

STRUCTURAL STEEL:

- 1. STEEL SHALL CONFORM TO THE FOLLOWING GRADES: ALL WF (U.N.O.): A992 GRADE 50 (FY=50) ALL ANGLE, BASE PLATES, CONN. PLATES (U.N.O.): A36 (FY=36) STRUCTURAL PIPE: A53 (FY=35) STRUCTURAL TUBE: A500 GRADE B (FY=46) HEADED STUDS: AWS D1.1, TYPE B THREADED ROD: A36 ANCHOR ROD: F1554, GR36 OR 55 PER DETAILS BOLTS: A325 / F1852 ELECTRODES: MATCHING STRENGTH, 70KS: MIN. 2. ALL STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE (A.I.S.C. 303-2010) CODE OF STANDARD PRACTICE, EXCEPT AS MODIFIED IN THESE NOTES AND THE PROJECT SPECIFICATIONS. 3. CONNECTIONS MAY BE BOLTED OR WELDED. THE FABRICATOR IS RESPONSIBLE FOR THE DESIGN, OF CONNECTIONS NOT DESIGNED ON THE DRAWINGS. GENERALLY, CONNECTIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE SCHEMATIC AND ARE ONLY INTENDED TO SHOW THE RELATIONSHIP OF MEMBERS CONNECTED. ANY CONNECTION THAT IS NOT SHOWN OR IS NOT COMPLETELY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY AN ENGINEER, REGISTERED IN THE STATE OF MISSOURI, RETAINED BY THE FABRICATOR. COMPLETELY DETAILED MEANS THE FOLLOWING INFORMATION IS SHOWN ON THE DETAIL: A. ALL PLATE DIMENSIONS AND GRADES. B. ALL WELD SIZES, LENGTHS, PITCHES, AND RETURNS. C. ALL HOLE SIZES AND SPACINGS. D. NUMBER AND TYPES OF BOLTS: WHERE BOLTS ARE SHOWN BUT NO NUMBER IS GIVEN, THE CONNECTION HAS NOT BEEN COMPLETELY DETAILED. E. WHERE PARTIAL INFORMATION IS GIVEN, IT SHALL BE THE MINIMUM REQUIREMENT FOR THE CONNECTION. SIGNED AND SEALED DESIGN CALCULATIONS FOR ALL PRIMARY BRACING AND HANGER CONNECTIONS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. 4. CONNECTION DESIGN FORCES: A. BEAMS, GREATER OF: 1. 55% OF TOTAL ALLOWABLE UNIFORM LOAD CAPACITY FROM A.I.S.C. 13TH EDITION TABLES FOR ALLOWABLE LOADS ON BEAMS, WOL 2. REACTIONS SHOWN ON DRAWINGS ARE ALLOWABLE LOADS. 3. 10 KIPS. B. MOMENT CONNECTIONS INDICATED ON THE DRAWINGS THUS: DESIGN FOR MOMENT SHOWN OR, IF NOT SHOWN, DEVELOP MOMENT CAPACITY OF MEMBER WITH fb = 0.66FY. C. MAINTAIN TENSION CAPACITY OF COLUMNS, DIAGONALS AND MEMBERS SUBJECT TO TENSION AT BOLT HOLES, NOTCHES, OR COPEES. D. CONNECTION FORCE NOTATION: P = AXIAL FORCE IN KIPS: (+) TENSION, (-) COMPRESSION V OR | ] = SHEAR IN KIPS M = MOMENT IN FOOT KIPS T = TORSION IN FOOT KIPS 1. LOADS SHOWN INCLUDE COMPENSATION FOR CODE PERMITTED STRESS INCREASES AND LOAD REDUCTIONS FOR CONNECTION DESIGN. 5. THE MINIMUM PLATE THICKNESS SHALL BE 3/8. 6. BOLTED CONNECTIONS: A. MINIMUM BOLT DIAMETER = 3/4" B. SLIP CRITICAL CONNECTIONS OF A325SC OR A490SC BOLTS SHALL BE USED FOR ALL BOLTED CONNECTIONS OF BRACING MEMBERS, MOMENT CONNECTIONS, CANTILEVERS, AND AS SHOWN ON THE DRAWINGS. OVERSIZED AND LONG-SLOTTED HOLES ARE ALLOWED FOR FRICTION CONNECTIONS. C. ALL OTHER BOLTED CONNECTIONS SHALL BE BEARING TYPE USING A325N OR A490N BOLTS. OVERSIZED HOLES AND LONG-SLOTTED HOLES ARE NOT ALLOWED UNLESS SHOWN ON THE DRAWINGS. D. A307 BOLTS MAY BE USED WHERE INDICATED ON THE DRAWINGS. E. PROTRUDING BOLT HEADS, SHAFTS OR NUTS SHALL NOT EXTEND INTO NOR PROHIBIT THE APPLICATION OF ARCHITECTURAL FINISHES AND THEY SHALL NOT EXTEND INTO NOR PROHIBIT THE PLACEMENT OF STEEL DECKING TO THE CORRECT LINE AND ELEVATION. F. THE FABRICATOR IS RESPONSIBLE FOR VERIFYING THE TENSION CAPACITY OF AXIALLY LOADED MEMBERS AFTER A SECTION IS REDUCED FOR BOLT HEADS. MEMBER SIZE MAY BE INCREASED OR CONNECTION PLATES ADDED AS REQUIRED. G. SHOP DRAWINGS SHALL INDICATE THE TYPE OF BOLT USED IN EACH CONNECTION AND THE ALLOWABLE VALUES USED FOR THE VARIOUS BOLT TYPES. 7. WELDED CONNECTIONS: A. WELDS ARE CONTINUOUS UNLESS NOTED OTHERWISE. B. ALL FILLET WELDS: A.I.S.C. MINIMUM BUT NOT LESS THAN 1/8" UNLESS NOTED OTHERWISE. C. ALL WELDING SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE" (A.W.S. D1.1-10) PUBLISHED BY THE AMERICAN WELDING SOCIETY. ELECTRODES FOR WELDING SHALL COMPLY WITH THE REQUIREMENTS OF TABLE 3.1 OF (A.W.S. D1.1-10). D. ALL GROOVE WELDS SHALL BE COMPLETE PENETRATION UNLESS NOTED OTHERWISE. 8. SPLICING OF STEEL MEMBERS, UNLESS SHOWN ON THE DRAWINGS, IS PROHIBITED WITHOUT WRITTEN APPROVAL OF THE ARCHITECT. 9. NO CHANGE IN SIZE OR POSITION OF THE STRUCTURAL ELEMENTS SHALL BE MADE OF HOLES, SLOTS, CUTS, ETC., AND ARE NOT PERMITTED THROUGH ANY MEMBER UNLESS THEY ARE DETAILED ON THE APPROVED SHOP DRAWINGS. 10. NO FINAL BOLTING OR WELDING SHALL BE MADE UNTIL AS MUCH OF THE STRUCTURE HAS BEEN PROPERLY ALIGNED AND WILL THEREBY BE STIFFENED. 11. UNLESS NOTED OTHERWISE, BEAMS SHALL BEAR 8" MINIMUM ON CONCRETE OR MASONRY. ANCHOR BEAMS TO MASONRY WITH A GOVERNMENT-TYPE ANCHOR. 12. FABRICATE ALL BEAMS WITH THE MILL CAMBER UP. 13. SHEAR STUDS: CONFORM TO A.W.S. D1.1-98, SHOP WELD EXCEPT WHERE APPLIED THROUGH METAL DECK. 14. REFERENCE SPECIFICATIONS FOR SPECIAL SOY BASED FINISH REQUIREMENTS

REINFORCED MASONRY:

- 1. NOTES APPLY TO MASONRY SHOWN ON STRUCTURAL DRAWINGS AND SHALL BE THE MINIMUM REQUIREMENTS FOR MASONRY SHOWN ON THE ARCHITECTURAL DRAWINGS. 2. CONCRETE MASONRY: A. COMPRESSIVE STRENGTH OF MASONRY, PRISM STRENGTH F'm = 2000 P.S.I. (MINIMUM) COMPRESSIVE STRENGTH OF CMU-2800 PSI. B. CONCRETE MASONRY UNITS: A.S.T.M. C90, GRADE N, TYPE 1 - 2 CELL UNITS LIGHTWEIGHT UNITS FOR INTERIOR WALLS. REGULAR WEIGHT UNITS FOR EXTERIOR WALLS. C. GROUT: A.S.T.M. C476-83 STRENGTH AT 28 DAYS = 125% OF F'm, 2500 P.S.I. MINIMUM. D. MORTAR: A.S.T.M. C270-89 TYPE - S, HYDRATED LIME REQUIRED 3. REINFORCED MASONRY REQUIRES CONTINUOUS SPECIAL INSPECTION - SEE SPECIFICATIONS. 4. REINFORCING BARS SHALL CONFORM TO A.S.T.M. A615, GRADE 60, UNLESS NOTED ON DRAWINGS. WIRES SHALL CONFORM TO A.S.T.M. A62.5. 5. DOWELS FROM C.I.P. CONCRETE SHALL MATCH THE VERTICAL REINFORCEMENT IN THE WALL ABOVE UNLESS NOTED OTHERWISE AND LAPPED BOTH ENDS. SUCH DOWELS SHALL BE FURNISHED BY THE CONCRETE CONTRACTOR. 6. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL BLOCK CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN 6 VERTICAL. DOWELS MAY BE GROUDED INTO A CELL IN VERTICAL ALIGNMENT, EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING. 7. SPLICED REINFORCING SHALL BE LAPPED AS NOTED BELOW. SPLICED BARS SHALL BE WIRED TOGETHER. #5 BARS, USE 3'-0" LAP #6 BARS, USE 5'-10" LAP #7 BARS, USE 7'-10" LAP MECHANICAL SPLICES ARE ACCEPTABLE AND SHALL DEVELOP 125% OF BAR STRENGTH. 8. VERTICAL BARS SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS EXCEEDING 192 DIAMETERS OF THE REINFORCING. 9. VERTICAL REINFORCING BARS SHALL HAVE A MINIMUM CLEARANCE OF 3/4 OF AN INCH FROM THE MASONRY OR ADJACENT BARS AND NOT LESS THAN ONE BAR DIAMETER BETWEEN BARS NOT SPLICED. 10. VERTICAL GROUTING MAY BE EITHER "LOW LIFT" OR "HIGH LIFT" AT CONTRACTOR'S OPTION. 11. VERTICAL CELLS THAT WILL BE GROUTED SHALL HAVE VERTICAL ALIGNMENT TO MAINTAIN A CONTINUOUS UNOBSTRUCTED CELL AREA NOT LESS THAN 2" x 3". 12. GROUTING SHALL BE STOPPED 1/2" BELOW THE TOP OF A COURSE SO AS TO FORM A KEY AT THE POUR JOINT. 13. GROUTING OF MASONRY BEAMS OVER OPENINGS SHALL BE DONE IN ONE CONTINUOUS OPERATION. 14. ALL BOLTS, ANCHORS, ETC., INSERTED IN THE WALLS, SHALL BE GROUDED SOLID INTO POSITION. 15. ALL HORIZONTAL REINFORCING (JOINT REINFORCING AND REBAR) SHALL CONTINUE AT CONTROL JOINTS. 16. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS AND DETAILS OF DOOR AND WINDOW OPENINGS FOR SPECIAL COURSING AND OTHER MASONRY DETAILS. THE INFORMATION SHOWN ON THE STRUCTURAL DRAWINGS IS INTENDED TO DEFINE THE STRUCTURAL REQUIREMENT ONLY. 17. PROVIDE VERTICAL WALL REINFORCING, SAME SIZE AS ADJACENT BAR, AT: CORNERS, ENDS, JAMBS, EACH SIDE OF OPENING, AND EACH SIDE OF CONTROL AND EXPANSION JOINTS. 18. CONTINUE REINFORCING THROUGH CONSTRUCTION JOINTS, AND AROUND CORNERS UNLESS NOTED OTHERWISE. 19. PROVIDE STANDARD HOOKS ON BARS TERMINATING INTO MASONRY FACE: - IN WALLS AT OPENINGS, HEADS, JAMBS, EXPANSION JOINTS, ENDS; - IN BEAMS AT TOP, BOTTOM, AND ENDS. 20. SPLICE CONTINUOUS TOP BARS AT MID SPAN AND BOTTOM BARS OVER SUPPORT. SPLICE VERTICAL REINFORCING AT FLOOR OR ROOF LINES. 22. COORDINATE BLOCKOUTS, REVEALS, HOLES, OPENINGS, AND BUILT IN ITEMS WITH ALL CONTRACT DOCUMENTS AND TRADES. 24. GROUT CELLS SOLID AT: REINFORCING, BOND BEAMS, INSERTS, ANCHORS, ELEVATOR GUIDE RAILS, AND 24" BELOW BEARING POINT OF STEEL SECTIONS AND 12" TO EASIDE. 25. SEE MISC. NOTES FOR EPOXY / ADHESIVE ANCHORS.

METAL ROOF DECK:

- 1. METAL ROOF DECK SHALL COMPLY WITH THE REQUIREMENTS OF THE STEEL DECK INSTITUTE. 2. PROJECT SPECIFICATIONS SEE PLANS FOR DECK TYPES AND GAUGES. METAL ROOF DECK HAS BEEN DESIGNED TO FUNCTION AS A DIAPHRAGM FOR THE TRANSMISSION OF LATERAL LOADS. CONNECTION OF DECK UNITS TO EACH OTHER AND TO SUPPORTS SHALL BE DESIGNED BY THE DECK SUPPLIER CONSISTENT WITH THE DECK PROPERTIES AND MANUFACTURER'S RECOMMENDATIONS. 3. LAP DECK 4" MINIMUM AT SPLICES CENTERED ON SUPPORT. 4. DO NOT SUSPEND POINT LOADS FROM DECK INCLUDING HANGERS FOR: CEILINGS, PIPES, 4. DUCTS, EQUIPMENT, ETC.. CONTRACTOR INSTALLING SUCH POINT LOADS SHALL PROVIDE SUB-FRAMING TO TRANSFER LOAD TO STRUCTURE SUPPORTING DECK. 5. FABRICATE DECK UNITS IN LENGTHS TO SPAN THREE OR MORE SUPPORT SPACINGS. 6. MINIMUM YIELD STRENGTH = 33 K.S.I. 7. REFERENCE SPECIFICATIONS FOR TYPE AND GAUGE OF DECK. 8. DECKING MANUFACTURER SHALL COORDINATE SIZE AND LOCATION OF ROOF OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

CONCRETE SLABS ON METAL DECK:

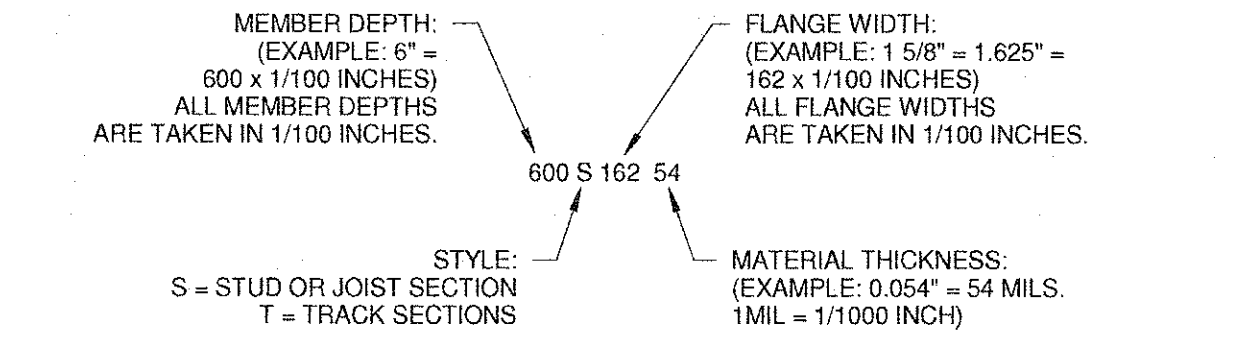
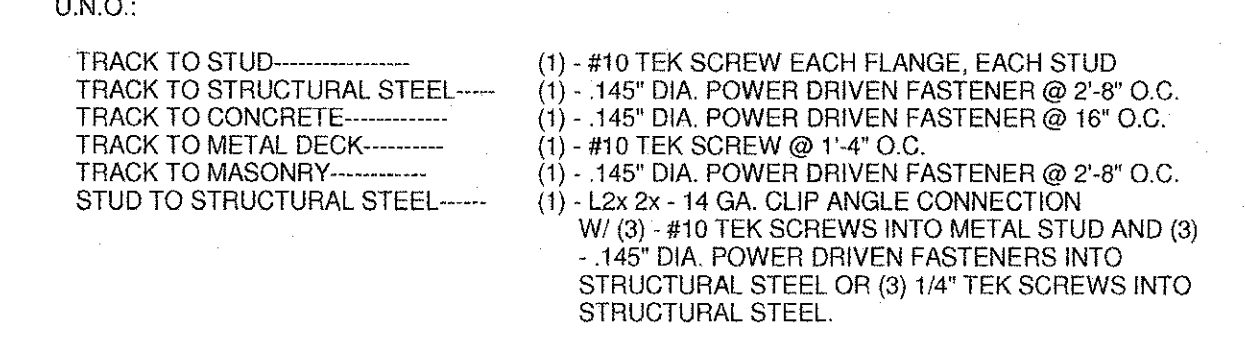
- 1. THE CONTRACTOR SHALL PLACE CONCRETE FOR SLABS SO THAT THE FINISH SURFACE IS SCREED LEVEL WITH AN ELEVATION WITHIN 1/4" OF THE TOP OF SLAB ELEVATION SHOWN ON THE PLANS. 2. SLABS SHALL HAVE A TROWELED FINISH AND AS A MINIMUM BE TRUE PLANES WITHIN 1/2" IN 10 FEET, AS DETERMINED BY A 10 FOOT STRAIGHTEDGE PLACED ANYWHERE ON THE SLAB IN ANY DIRECTION. 3. CONSIDERATION SHALL BE GIVEN TO SEQUENCING OF CONCRETE PLACEMENT SO AS TO CONTROL FINISH ELEVATIONS WITHIN THE SPECIFIED LIMITS. 4. CONSIDERATION SHALL BE GIVEN TO SEQUENCING OF CONCRETE PLACEMENT SO AS TO CONTROL FINISH ELEVATIONS WITHIN THE SPECIFIED LIMITS. NO MORE THAN 1/2" OF ADDITIONAL CONCRETE THICKNESS SHALL BE POURED TO MAINTAIN FLOOR FLATNESS OR LEVELNESS. IF SUCH A SITUATION OCCURS G.C. SHALL STOP POUR, LET DECK HARDEN AND FINISH WITH A TOPPING SLAB.

STEEL JOIST:

- 1. STEEL JOISTS SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST "S.J.I." SPECIFICATIONS. 2. BRIDGING SHALL BE SPACED IN ACCORDANCE WITH THE LATEST S.J.I. SPECIFICATIONS AND THE ERECTION DRAWINGS OF THE JOIST SUPPLIER. 3. UNLESS NOTED OTHERWISE, BRIDGING SHALL BE SPACED IN ACCORDANCE WITH THE LATEST S.J.I. SPECIFICATIONS. 4. STEEL JOIST BRIDGING SHALL BE PLACED AND JOIST ENDS FIXED PRIOR TO THE APPLICATION OF ANY LOADS. 5. MINIMUM BEARING REQUIREMENTS, UNLESS NOTED OTHERWISE K SERIES: 2 1/2" ON STRUCTURAL STEEL, 4" CONCRETE LH SERIES: 4" ON STRUCTURAL STEEL, 6" CONCRETE JOIST GIRDERS: 4" ON STRUCTURAL STEEL, 6" CONCRETE 6. JOISTS SHALL BE ATTACHED TO SUPPORTING STEEL WORK AS NOTED ON THE DRAWINGS. 7. AT COLUMN CENTERLINES, JOIST SHALL BE BOLTED TO STRUCTURAL STEEL BEAMS, WITH (2) BOLTS FOR ERECTION PURPOSES ONLY. ALL JOIST TO BE WELDED IN PLACE FOR FINAL CONNECTION UNLESS SPECIFIC DETAIL AT EXPANSION JOINT IS REFERENCED ON PLAN. 8. SEE DETAILS FOR ATTACHMENT OF JOISTS TO CONCRETE AND MASONRY. 9. BRIDGING THAT TERMINATES AT, OR IS INTERRUPTED BY, STRUCTURAL STEEL BEAMS, SHALL BE ATTACHED THERETO BY FIELD WELDING OR BOLTING. SEE DRAWINGS FOR DETAIL OF ATTACHMENT OF BRIDGING TO CONCRETE OR MASONRY. 10. JOIST SHALL BE STOCKPILED AT THE JOBSITE IN A VERTICAL POSITION, RESTING ON THEIR TOP OR BOTTOM CHORDS, AND SHALL BE ADEQUATELY SUPPORTED WITH WOOD BLOCKING. KEEP JOIST FREE OF MUD AND DIRT. 11. IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO SEE THAT JOISTS WHICH ARE DAMAGED, KINKED, BENT, OR WITH BROKEN WELDS, ARE NOT PLACED IN THE STRUCTURE. 12. JOIST SUPPLIER SHALL DESIGN JOISTS AND SUBMIT CALCULATIONS, AS REQUIRED BY THESE DRAWINGS AND SPECIFICATIONS. 13. THE JOIST DESIGN AND BRIDGING PLACEMENT SHALL BE CHECKED BY THE JOIST MANUFACTURER USING THE NET UPLIFT SPECIFIED ON THE DRAWINGS. CHANGES IN JOIST SIZE AND/OR BRIDGING PLACEMENT WILL SHOW UP ON THE SHOP DRAWINGS. 14. LOCATE PIPE AND EQUIPMENT HANGERS AND OTHER CONCENTRATED LOADS ONLY WHERE LOADS ARE SHOWN ON JOIST SHOP DRAWINGS. ATTACHMENT METHOD AS APPROVED BY JOIST MANUFACTURER. 15. ROOF JOIST AND JOIST GIRDER WELDS TO SUPPORTING STEEL WORK TO CONFORM TO THE FOLLOWING UNLESS NOTED OTHERWISE ON THE DWGS. JOIST SHOP DRAWINGS TO SHOW WELD SIZES AND LENGTHS ON THE DRAWINGS.

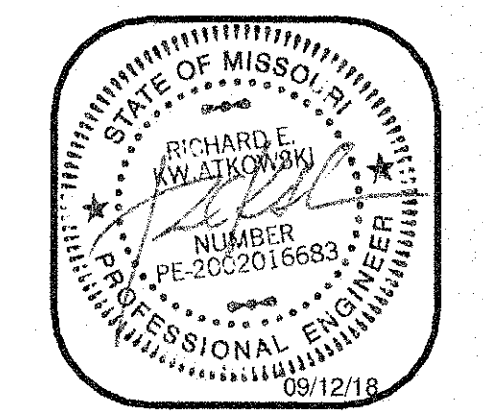
COLD-FORMED STEEL:

- 1. ALL SIZING BASED ON STEEL STUD MANUFACTURERS ASSOCIATION (CSBO ER-4943P) PRODUCT TECHNICAL INFORMATION. 2. MATERIALS SHALL CONFORM TO THE FOLLOWING: A. GALVANIZED MATERIAL: 1. ALL GALVANIZED STUDS AND JOISTS 12, 14 AND 16 GAUGE (97, 68 AND 54 MIL) SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE MINIMUM REQUIREMENTS OF ASTM A653 SS, GRADE 50, CLASS 1 OR 3 WITH A MINIMUM YIELD OF 50,000 PSI. 2. ALL GALVANIZED 18 AND 20 GAUGE (43 AND 33 MIL) STUDS AND JOISTS: ALL GALVANIZED TRACK, BRIDGING, END CLOSURES AND ACCESSORIES SHALL BE FORMED FROM STEEL THAT CORRESPONDS TO THE REQUIREMENTS OF ASTM A653 SS, GRADE 33 WITH A MINIMUM YIELD OF 33,000 PSI. 3. ALL GALVANIZED STUDS, JOISTS, TRACK, BRIDGING AND ACCESSORIES SHALL BE FORMED FROM STEEL HAVING A GALVANIZED COATING MEETING THE REQUIREMENTS OF ASTM A525. B. PROPERTIES: 1. THE PHYSICAL AND STRUCTURAL PROPERTIES LISTED BY THE STEEL STUD MANUFACTURER ASSOCIATION AND AISI DESIGN MANUAL SHALL BE CONSIDERED THE MINIMUM PERMITTED FOR ALL FRAMING MEMBERS. SPECIFICALLY, THE FOLLOWING MINIMUM PROPERTIES, CALCULATED IN ACCORDANCE WITH THE LATEST AISI SPECIFICATION SHALL BE PROVIDED: IX (IN 4), SX (IN 3), AREA (IN 2), RX (IN.), FY (KSI), RESISTING MOMENT (IN-LB.). C. SUBSTITUTIONS: 1. ANY SUBSTITUTIONS MUST BE APPROVED IN WRITING PRIOR TO DELIVERY, BY THE ARCHITECT AND/OR ENGINEER OF RECORD. 3. INSTALLATION OF STUDS SHALL BE AS PER ASTM C1007-00 "INSTALLATION OF LOAD BEARING (TRANSVERSE AND AXIAL) STEEL STUDS AND ACCESSORIES", ASTM C955-00a "SPECIFICATION FOR LOAD BEARING (TRANSVERSE AND AXIAL) STEEL STUDS, RUNNERS (TRACK), AND BRACING OR BRIDGING FOR SCREW APPLICATION OF GYPSUM BOARD AND METAL PLASTER BASES", AND ASTM C754-00 "SPECIFICATION FOR INSTALLATION OF STEEL FRAMING MEMBERS TO RECEIVE SCREW ATTACHED GYPSUM BOARD". 4. ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS, OR AS REQUIRED FOR AN ANGULAR FIT AGAINST ABUTTING MEMBERS. MEMBERS SHALL BE HELD POSITIVELY IN PLACE UNTIL PROPERLY FASTENED. 5. ALL TRACK BUTT JOINTS, ABUTTING PIECES OF TRACK SHALL BE SECURELY ANCHORED TO A COMMON STRUCTURAL ELEMENT, OR THEY SHALL BE BUTT-WELDED OR SPLICED TOGETHER. 6. ALL STUD BRIDGING SHALL BE ATTACHED IN A MANNER TO PREVENT STUD ROTATION. BRIDGING ROWS SHALL BE SPACED ACCORDING TO DIETRICH INDUST. RECOMMENDATION. 7. TEMPORARY BRACING SHALL BE PROVIDED UNTIL ERECTION IS COMPLETED. 8. STUD ENDS MUST BE SQUARELY SEATED AGAINST THE TRACK WEB. BOTH STUD FLANGES MUST BE ATTACHED TO TRACK MEMBERS AT TOP AND BOTTOM. 9. STUD BRIDGING SHALL BE PROVIDED BY 1-1/2" COLD ROLLED U-CHANNEL. THE U-CHANNEL MUST BE ATTACHED TO EACH STUD BY WELDING OR ATTACHING WITH CLIP ANGLES AND SCREWS. HORIZONTAL STRAPPING AND SOLID BRIDGING WITH TRACK MEMBERS CAN ALSO BE USED FOR BRIDGING. BRIDGING SHALL BE SPACED AT 40" O.C. MAX. THE FOLLOWING MINIMUM COLD FORMED STEEL ATTACHMENTS SHALL BE PROVIDED U.N.O.:



SPECIAL INSPECTION:

- 1. PERIODIC INSPECTION OF STEEL FABRICATOR'S SHOP FOR QUALITY CONTROL AND FABRICATION PROCESSES THAT COMPLY WITH AISC CODE OF STANDARD PRACTICE. 2. PERIODIC INSPECTION OF BOLTED AND WELDED STEEL CONNECTIONS IN THE FIELD PER TABLE 1704.3 OF THE 2015 INTERNATIONAL BUILDING CODE. 3. PERIODIC INSPECTION OF CONCRETE MATERIALS, REINFORCING AND PLACEMENT SHALL BE INSPECTED PER TABLE 1704.4 OF THE 2015 INTERNATIONAL BUILDING CODE. 4. CONTINUOUS INSPECTION OF MASONRY MATERIALS, REINFORCING & PLACEMENT SHALL BE INSPECTED PER TABLE 1704.5.3 OF THE 2015 INTERNATIONAL BUILDING CODE. 5. INSPECTION OF SITE SOILS, FILL PLACEMENT, AND BEARING CAPACITIES BY A LICENSED GEOTECHNICAL ENGINEER AS FOLLOWS: A. OBSERVATION OF PROOF ROLLING FOR THE SITE PRIOR TO FILL PLACEMENT. COMPACTION TESTING OF STRUCTURAL FILL PLACEMENT. LIFTS SHALL NOT EXCEED 8" B. COMPACT STRUCTURAL FILL TO 95% DRY DENSITY AS MEASURED BY THE STANDARD PROCTOR METHOD, ASTM D-698 C. PROVIDE BEARING TESTS AT EACH FOOTING LOCATION TO CONFIRM BEARING CAPACITY. 6. SPECIAL INSPECTION IS NOT A SUBSTITUTE FOR INSPECTION BY A CITY/COUNTY INSPECTOR. 7. SPECIAL INSPECTED WORK WHICH IS INSTALLED OR COVERED WITHOUT APPROVAL OF LOCAL AND SPECIAL INSPECTORS IS SUBJECT TO REMOVAL OR EXPOSURE. 8. SPECIAL INSPECTORS MUST BE CERTIFIED BY THE CITY/COUNTY TO PERFORM THE TYPES OF INSPECTIONS SPECIFIED. 9. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO INFORM THE SPECIAL INSPECTOR OR INSPECTION AGENCY AT LEAST ONE WORKING DAY PRIOR TO PERFORMING ANY WORK THAT REQUIRES SPECIAL INSPECTION. ANY WORK PERFORMED WITHOUT REQUIRED SPECIAL INSPECTION IS SUBJECT TO REMOVAL. 10. SUBMIT WRITTEN REPORTS WITHIN TWO DAYS OF TESTING TO ENGINEER OF RECORD.



RICHARD E. KWIAWKOWSKI MO LIC# PE-2002016683 EXP: 12/31/18

CENTRAL MISSOURI PROFESSIONAL SERVICES, INC. CIVIL ENGINEER MISSOURI STATE CERTIFICATE OF AUTHORITY # SSC ENGINEERING, INC. MECHANICAL/ELECTRICAL/PLUMBING ENGINEER MISSOURI STATE CERTIFICATE OF AUTHORITY #001944 SSC ENGINEERING, INC. STRUCTURAL ENGINEER MISSOURI STATE CERTIFICATE OF AUTHORITY #001244



STRUCTURAL GENERAL NOTES

MISSOURI SOYBEAN INNOVATION CENTER NEW OFFICE BUILDING 734 S. COUNTRY CLUB DRIVE JEFFERSON CITY, MISSOURI

PROJECT NO. 1613 DATE: SEPT. 12, 2018

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